

Research Article

A “Non-Invasive” Technique for Qualifying the Reinforced Concrete Structure

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In recent years, a lot of studies on built heritage emphasize the need to use appropriate techniques to evaluate the current condition of the structure before designing an intervention. The research focuses on the restoration of reinforced concrete buildings that begin to show signs of decay and deterioration. To verify the state of a building, it's possible to use the “destructive” methods (that require a local removal of material) and “nondestructive” tests. The combined results from different “nondestructive” tests are very interesting instruments to assess the concrete strength. This methodological approach can help to reduce the possible errors when using the sclerometer and ultrasonic tests separately; in this way, the combined method called “SonReb” (SONic + REBound) was developed. This paper would highlight the importance with respect to cultural heritage buildings and on the studied structure and contribute to developed engineering strategies to maintenance and restoration. The above-defined methodology has been tested on a postwar building which is located in Gravina in Puglia (Italy): the “Centrone” theatre; it was built using a mixed structure, that is, reinforced concrete and bearing masonry built of local stone. The building was used until the 1990s and now is abandoned. The analysis and qualification of the masonry structures of built heritage show how this approach is useful for classifying the pathological events on each building and to implement the innovative solutions to improve the durability of a restoration intervention.

1. Introduction

In the last decades, the architectural heritage of the modern movement seems to be more at risk than during any other period. This built inheritance embodies the dynamic spirit of the industrial age. At the end of the 1980s, many modern masterpieces had already been demolished or changed beyond recognition. This was mainly due to the fact that many were not considered to be elements of heritage that their original functions have substantially changed and that their technological innovations have not always endured long-term stresses.

A thorough research of built heritage enables understanding of the evolution of design philosophies and underlying cultural meanings and messages, artistic and functional qualities, and engineering achievements. A detailed knowledge of building materials, construction techniques, environmental services, external impacts, and internal impacts

of use or disuse assist in identifying the problems affecting buildings and defining a methodological approach for interventions.

The research objective is to provide new qualitative information on the strength of reinforced concrete structures of a building by using innovative, non-invasive testing techniques. The case of study is the “Centrone” theatre in Gravina in Puglia (Italy).

The confluence of the local architectural styles (vernacular and academic) and the emerging aesthetic of reinforced concrete is explored to outline the context that influenced the building design.

The research methodology includes (a) the context in which the buildings were designed, (b) their history, (c) building technologies used, (d) non-invasive testing of the reinforced concrete structures, (e) the analysis of the test results, and (f) the conclusions.

Testing methods could be “destructive”, as they require a local removal of material, or “nondestructive”, that is, they do not affect the structure. A sclerometer test, an ultrasonic test and their combined use, called SonReb (*SONic* + *REBound*), are “nondestructive” tests on reinforced concrete. The combined tests are a very useful method for assessing the concrete strength and to reduce the possibility of errors that can happen if the tests are not combined, as it has been noticed that the humidity content of a structural element can influence the sclerometer index and the ultrasound speed [1]. The combined method requires shorter time to obtain the results.

2. A “Critical” Approach to Building Restoration

The conservation intervention on a historic building, regardless of its architectural and/or artistic value, is generally more appropriate if information on its construction, evolution to date, materials, construction techniques, and structure is available. The conservation of built heritage highlights different issues such as the vast number of buildings needing attention and the urgency of cases that have to be resolved with limited economic resources and time.

To undertake a suitable intervention, three questions have to be answered: whether, where, and how to restore. The fourth question could be added, in which the economic aspect dominates: when to restore. To be able to respond adequately to these questions, it is necessary to proceed by developing specifications step by step, starting from a detailed knowledge of the entire building, the level and causes of degradation, and by finishing with the “operational” description of the proposed interventions.

Within the preliminary data acquisition phase, the direct survey of the building characteristics and condition and the mapping of any noticed changes are accompanied by the research on the project documentation and the events that have affected the structure during its construction and throughout the building life.

These approaches, often coupled with normative models which translate living conditions into objective parameters and standards, hinder the interventions such as a “simple” maintenance or a restoration or produce the result that is not logically related to the structural, typological, functional, and technological characteristics of the artefacts.

An “appropriate” restoration should plan the reuse of built heritage and aim to achieve building performance comparable to new buildings. The reuse interventions should be integrated with the conservation and not imposed.

The above methodology highlights how the approach could be helpful for the classification of pathological events within a building and for the application of innovative solutions to increase the durability of restoration interventions.

3. The Investigative Techniques for Concrete

The investigative techniques for concrete are also classified in two defined macrocategories (“destructive” and “non-destructive”). The former are based on the extraction of

concrete samples to undertake compressive tests and represent the most reliable instrument for assessing the mechanical properties of concrete. The second investigative typology, the nondestructive tests, can be further subdivided in: really “nondestructive” investigations and “partially destructive” investigations. The latter include (a) the penetration test with a Windsor gun (ASTM C 83) which enables the identification of compressive resistance of concrete by measuring the depth of penetration of the special metal pins projected with a Windsor gun into concrete, (b) the extraction test (pull-out) (UNI 10157:1992-ASTM C 900-06) which enables the assessment of compressive resistance of concrete by measuring the force used by a hydraulic jack for extracting a special plug inserted into concrete.

The really nondestructive investigations include, among others, (a) endoscopy that enables a direct observation of form and appearance of an investigated object, (b) thermography that assists in recognising potential structural anomalies by using the capacity of materials to transfer heat, (c) magnetometry which enables localising metal bars in reinforced concrete, (d) the Ground Penetrating Radar (GPR), especially with regard the investigation of the internal reinforcement bars, (e) measurement of the electric potential of concrete which enables defining the level of corrosion of metal reinforcements in concrete, (f) ultrasound investigation that allows qualitative assessment of the concrete resistance by using the capacity of the concrete components to transfer ultrasound waves, (g) sclerometric tests that assess the concrete resistance by reading the bounce results, and finally, (h) so-called “SonReb” (*SONic* + *REBound*) that enables assessing the concrete resistance by combining the speed of ultrasound waves and the index of surface bounce through a synergic use of the two previous investigations.

3.1. The “SonReb” Method for Determining the Resistance of Concrete. The SonReb method, as mentioned above, allows a qualitative determination of the concrete resistance [2] through the cross-examination of the values of the speed of ultrasound waves and the values of sclerometric bounce. This investigation method is standardised by RILEM Recommendations [3] 43 CND-EN 13791:2007, the EC regulation 1-2010 UNI EN 12504-2:2001, ASTM C597, UNI EN 12504-4:2005, the Test Report CUR 69, the standards UNI 7997, UNI 9524 and UNI 83308.

This method is used for assessing the resistance of concrete, enabling the elimination of errors, at least partly, that appear when the two investigation methods are separately applied. This method, in fact, allows reducing the errors made when the sclerometric and ultrasound tests are undertaken separately [4].

It has been noticed, for example, that the humidity content leads to an underestimation of the sclerometric index and in an inversely proportional way leads to an overestimation of the ultrasound speed; similarly, the sclerometric index rises in a directly proportional way to the increase of the age of concrete [1], while the ultrasound speed decreases in an inversely proportional way to it [5].

In fact, the risk that can come up in the separate use of sclerometric and/or ultrasound tests is related, for example,

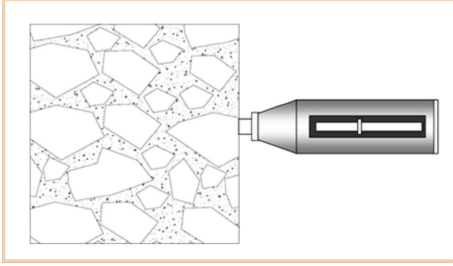


FIGURE 1: Sclerometer beats against a large piece of aggregate.

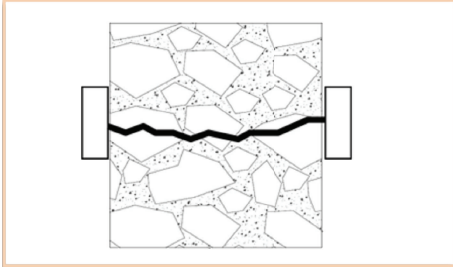


FIGURE 2: Ultrasound equipment: the impulse transmission.

to the chance nature of the position of aggregates in relation to the external surface of an investigated component; the sclerometer can easily beat against a large piece of aggregate, probably obtaining a higher value of the bounce index S in relation to the one returned when the blow is executed on a homogenous concrete surface (Figure 1).

Again as an example, the ultrasound test (Figure 2) can equally be affected by the chance nature of the disposition of aggregates in a cement mix when the gaps between the aggregates are arranged in the way that induces the rise of the void index; in this case, the speed of wave spread V decreases in relation to the value that would be obtained when the wave spreads through an area of “homogenous” concrete.

The application of the “SonReb” method asks for the appraisal [6] of local values of the ultrasound speed V and of the bounce index S from which it is possible to obtain the resistance of concrete R_c through expressions such as:

$$R_{c,sonReb} = a \cdot S^b \cdot V^c. \quad (a)$$

In the scientific literature this formula has assumed different forms, each one expressing the experiments undertaken directly on site or in a laboratory on standardised samples.

For example:

(i) Gasparik [7] (1992)

$$R_{c,2} = 8,06 \cdot 10^{-8} \cdot S^{1,246} \cdot V^{1,85}, \quad (b)$$

(ii) RILEM (1993) [2], NDT 4

$$R_{c,1} = 9,27 \cdot 10^{-11} \cdot S^{1,4} \cdot V^{2,6}, \quad (c)$$

(iii) Di Leo and Pascale [6] (1994)

$$R_{c,3} = 1,2 \cdot 10^{-9} \cdot S^{1,058} \cdot V^{2,446}, \quad (d)$$

in which:

- (i) R_c is the resistance of a cube under compression [N/mm²];
- (ii) S is the sclerometric index;
- (iii) V is the ultrasound speed [m/s].

The formula (c) depicts the correlation curve applied in the investigated case studies. In (c) R_c is expressed in MPa and the ultrasound speed in m/s; this relationship is related to a standard concrete whose properties are described in RILEM 43-CND. When a different type of concrete is employed the following relationship is applicable [8]:

$$R_{sonReb} = R'_{sonReb} \cdot (C_c \cdot C_d \cdot C_a \cdot C_f \cdot C_p \cdot C_m), \quad (1)$$

where R'_{sonReb} is the value obtained from (1), while C_c (cement type), C_d (cement content), C_a (aggregate types), C_f (proportions of fines), C_p (maximum aggregate size), and C_m (errors) are coefficients of influence that permit the extension of (b) to the cases of a nonstandard concrete (as defined above).

Hence, if the concrete has the same characteristics as the one whose experimental curves are available, the graph directly provides the estimated resistance of concrete. Otherwise, as usually happens, when the concrete has a different composition from the one presented by the curve, the corrective coefficients, that take into account the type of cement and the related dosage, the nature and dimensions of aggregates, and the potential additives need to be applied to obtain an approximately true estimate of the concrete resistance [1].

It is evident that the above equations could not have universal validity except for the fact that the values of S and V depend on the characteristics of concrete, even when the specific indications on the limitations of applicability are missing.

However, a qualitative appraisal of the resistance of concrete can be made even by using the graphs (Figure 3) which show a series of isoresistance curves in the plane V - S (obtained from the above analytic expressions) and which refer to the tests undertaken on standardised samples in a laboratory.

Although it appears absolutely necessary to analyse a wider range of cases, the above methodological investigation approach, based on the comparative analysis of the two described test campaigns, suggests several important considerations regarding the modality of investigation and the interpretation of results, demonstrating the need for establishing general investigation criteria: more than defining in a strict manner the number of tests that should be undertaken, it would be necessary to preset the level of significance to be achieved.

4. The Case of Study: “Centrone” Theatre in Gravina in Puglia (Italy)

The above-defined methodology has been tested on a postwar building which is located in Gravina in Puglia

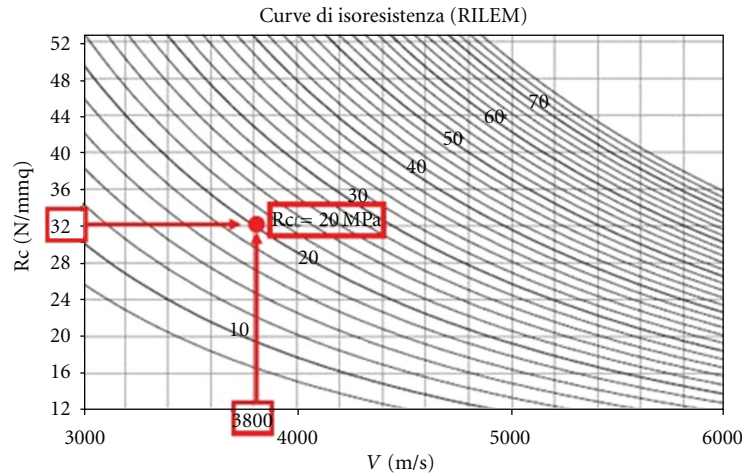


FIGURE 3: Example of iso-resistance curves—RILEM NDT 4 recommendations.



FIGURE 4: Historical image of vault construction.



FIGURE 5: Historical image of iron trusses.

(Italy): the “Centrone” theatre; it was built using a mixed structure, that is, reinforced concrete and bearing masonry built of local stone.

The building is not very old, but it is very interesting for the local civic history; in fact, it represents different values. Values that are set in different reels from the life of the building and revolved the surrounding environment in which it is inserted.

Historical value related to the historical development of the building and to the importance that it has in terms of expression of the local civic history; social value that is related to the social development of the city; cultural value that is related to the cultural growth of society; value of identity that has the building in terms of physics and metaphysics belong to a place then; but not least; the economic value related to the ability to reuse the building readapting it to new needs.

The building was built between 1946 and 1948; the foundations were built on pillars and vaults (Figure 4) and the pitched roof using iron trusses (Figure 5).

The building is a rectangular block (Figure 6), developed on three levels and covered using plaster and stone; the main façade is symmetrical, tripartite and follows the ground inclination; in the opposite side, the building shows two projecting polygonal elements realized using the local stone.

In the central part of the building; approximately 10 m wide; is located the entrance, highlighted by the massive



FIGURE 6: The “Centrone” theatre in Gravina in Puglia (Italy).

presence of two Doric columns that support a loggia. The building is organized in different spaces: the central part, where at ground floor there are the foyer entrance and ticket office, while on the first floor there is the Room “Italy” (Figure 7) that contains 110 seats; at second floor there are three flats for the managers of the theatre. The mail hall contains the stalls with 800 seats (a big space 32.5 m long and 21 m wide); it is partly covered by a balcony (at the first level) with 300 seats and a gallery on the second level with 300 seats.

The pathologies that interest the building are not particularly serious, since it does not affect in any way the



FIGURE 7: The room “Italy” on the first floor of the theatre.

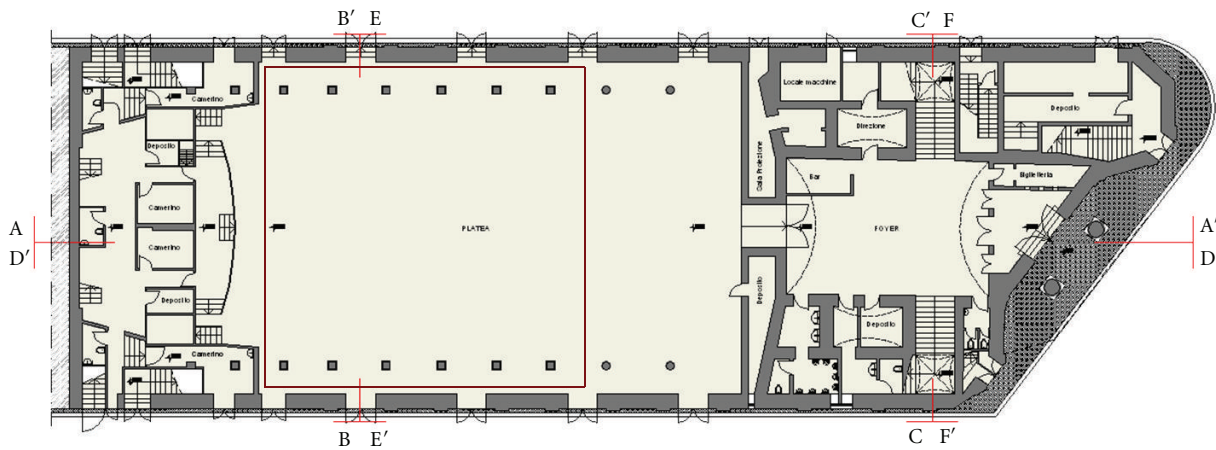


FIGURE 8: The specification of the location of the investigation points.

static system of structure and are easily solvable with non-invasive interventions. Inside are not detectable serious and manifest diseases, except for some water infiltrations from the roof that are determined as chromatic alterations and plaster detachment.

The condition of the pillars is generally acceptable, since they do not show obvious pathologies; however, you should note that some have a bad state of preservation of the plaster due to moisture that caused severe swelling, internal cavities and an evident surface lack of homogeneity.

4.1. On-Site Tests: Formulation. Essential elements for the organisation of an investigation campaign are the selection of the components that will be examined (which have to be representative of the whole structure), the investigation methodology that will be used, and the number and location of the investigation points; these requirements, in fact, are fundamental to guarantee a certain level of “reliability” of the obtained results and a “trustworthiness” with regard

to the qualitative indications related to the characteristics of the material. In the case of the “Centrone” theatre, the identification of the components to be investigated was made by selecting the structural components which make the internal core of the load-bearing structure for two reasons: (1) because they enable to investigate a concrete in the state of “natural aging” (i.e., without considering the aggressive atmospheric agents which could have modified the condition of the material) and (2) for easiness of selecting the testing points. The structure of the whole building consists of a dual typology of structural components: those inserted in the external fabric (therefore difficult to investigate) and those located within the building.

The selection of the location of the investigation points (Figure 8) was random (with the aim to guarantee the representativeness of the investigation for the entire structure); in addition, “homogenous” areas [1] (which have the same characteristics) were noted and considered by making the obtained results “qualitative” and representative for all the

TABLE 1: The value of rebound index.

Structural element		Sclerometer value										Average value
Element code	Size [cm]	Rebound value										
P1 Dx T	43 × 43	27	28	26	29	25	25	31	26	28	28	28
P2 Dx T	43 × 43	34	38	36	42	40	36	37	38	41	38	38
P3 Dx T	43 × 43	32	33	30	31	32	32	34	31	31	31	32
P4 Dx T	43 × 43	35	33	34	30	36	34	34	34	36	38	38
P5 Dx T	43 × 43	35	33	34	34	33	32	35	37	34	34	34
P6 Dx T	43 × 43	35	37	38	38	36	34	35	38	36	36	36
P1 Sx T	43 × 43	41	37	45	47	37	38	42	39	40	41	41
P2 Sx T	43 × 43	43	42	42	40	44	42	46	44	44	40	43
P3 Sx T	43 × 43	34	34	37	35	34	33	34	32	36	35	34
P4 Sx T	43 × 43	32	28	32	32	34	34	34	34	30	32	32
P5 Sx T	43 × 43	30	28	28	32	30	28	32	32	30	30	30
P6 Sx T	43 × 43	34	35	34	36	35	37	38	35	38	35	36
P1 Dx I p	43 × 43	23	21	21	22	22	25	25	26	24	22	23
P2 Dx I p	43 × 43	33	32	33	32	30	30	30	33	31	30	31
P3 Dx I p	43 × 43	37	40	38	41	43	38	38	38	41	38	39
P4 Dx I p	43 × 43	38	38	38	38	37	39	37	38	39	39	38
P5 Dx I p	43 × 43	37	39	35	38	37	38	38	40	38	40	38
P6 Dx I p	43 × 43	36	38	37	37	38	38	37	38	36	38	37
P1 Sx I p	43 × 43	26	22	24	22	24	20	23	22	22	22	23
P2 Sx I p	43 × 43	40	41	37	40	40	39	38	38	37	38	39
P3 Sx I p	43 × 43	40	37	35	37	36	35	35	35	35	35	36
P4 Sx I p	43 × 43	35	35	35	37	35	37	36	38	36	38	36
P5 Sx I p	43 × 43	40	40	37	35	36	35	38	36	36	36	37
P6 Sx I p	43 × 43	36	38	36	38	38	38	40	40	38	38	38

components that show the same characteristics; a further condition is that the selected surfaces do not show any obvious condition of degradation. The methodology used to undertake the tests was exclusively selected in terms of the possibility to undertake the tests. Regarding the sclerometric tests, they were undertaken by placing the instrument always orthogonally in relation to the investigated surface; the preference was given to the direct method of investigation. The considered reference (in terms of the number of tests with the aim to guarantee the reliability of the results) for the investigation campaign was taken from the framework of the Italian investigation procedures.

4.2. On-Site Tests: Analysis and Results. Sclerometer tests were carried out following the directions contained in UNI EN 12504-2 (2001). The instrument used is the concrete GEI model. On each pillar identified, it was carried out 10 measurements (Table 1) of the value of rebound (for each test, ten values were taken, from which the average value was considered as illustrative).

Three different areas were selected for each investigated component; at 0.70 m, at 1.40 m, and at 2.10 m from the floor. This selection, in fact, was motivated by the need to investigate the columns at the points of major stress (base and middle), having assumed (1) a uniform distribution of

the loads and imposed loads (due to the homogenous structure) and (2) a homogenous condition of the loads (there are no visible cracks to induce any different considerations).

Ultrasonic tests were carried out according to the UNI EN 12504-4 (2005). Ultrasonic equipment used is the DSP model of Ultrasonic UTD 1004. On each pillar identified, it was carried out 10 measurements (Table 2) of the value of rebound (for each test ten values were taken, from which the average value was considered as illustrative).

These data were compared with the method “SonReb” (Tables 3, 4, 5, and 6) to improve—as said before—the qualitative interpretation of results; the analysis showed satisfactory results of the state of concrete condition, as well as the importance to compare the ultrasonic and sclerometer tests.

The obtained results show a significant homogeneity of values measured at each investigated level; more precisely, the sclerometric tests show an average value (of the ten measurements made at each point) of the bounce index uniformly distributed on the surfaces; whereas, the ultrasound tests returned average values of the ultrasound between a minimum value of 3050 m/s² and a maximum value of 3660 m/s². Comparing the results by using the method SonReb, the obtained values of the concrete resistance are from a minimum value of 18 MPa to a maximum value of

TABLE 2: The value of ultrasonic speed.

P1 Sx T average ultrasonic speed: 2862,9 m/s						
Height: 0,70 m Size 0,46 × 0,46	X direction	2039	2036	2035	2038	2041
	Y direction	2032	2034	2045	2048	2029
Height: 1,40 m Size 0,43 × 0,43	X direction	3656	3669	3686	3676	3664
	Y direction	3668	3672	3675	3663	3665
Height: 2,10 m Size 0,40 × 0,40	X direction	2043	2041	2042	2045	2046
	Y direction	2049	2068	2061	2043	2048
Height: 2,10 m Size 0,40 × 0,40	X direction	3665	3681	3687	3690	3684
	Y direction	3689	3676	3679	3695	3700
Height: 2,10 m Size 0,40 × 0,40	X direction	2052	2045	2049	2046	2048
	Y direction	2058	2060	2039	2059	2056
Height: 2,10 m Size 0,40 × 0,40	X direction	3685	3684	3682	3679	3669
	Y direction	3689	3694	3692	3686	3696
P1 Dx T average ultrasonic speed: 2873,2 m/s						
Height: 0,70 m Size 0,46 × 0,46	X direction	3367	3355	3362	3360	3355
	Y direction	3394	3397	3389	3362	3392
Height: 1,40 m Size 0,43 × 0,43	X direction	2520	2521	2531	2513	2512
	Y direction	2515	2510	2533	2511	2525
Height: 1,40 m Size 0,43 × 0,43	X direction	3250	3257	3260	3252	3247
	Y direction	3302	3315	3333	3341	3348
Height: 2,10 m Size 0,40 × 0,40	X direction	2490	2499	2510	2512	2505
	Y direction	2496	2493	2495	2491	2510
Height: 2,10 m Size 0,40 × 0,40	X direction	3322	3320	3333	3327	3325
	Y direction	3139	3152	3134	3152	3157
Height: 2,10 m Size 0,40 × 0,40	X direction	2526	2516	2545	2544	2558
	Y direction	2556	2559	2562	2568	2564
P6 Sx T average ultrasonic speed: 2134,1 m/s						
Height: 0,70 m Size 0,46 × 0,46	X direction	2160	2084	2053	2016	2066
	Y direction	2161	2016	2103	2105	2150
Height: 1,40 m Size 0,43 × 0,43	X direction	2160	2154	2111	2102	2013
	Y direction	2163	2136	2165	2130	2122
Height: 1,40 m Size 0,43 × 0,43	X direction	2160	2136	2165	2154	2155
	Y direction	2155	2153	2151	2160	2165
Height: 2,10 m Size 0,40 × 0,40	X direction	2152	2154	2150	2153	2103
	Y direction	2100	2155	2015	2140	2150
Height: 2,10 m Size 0,40 × 0,40	X direction	2150	2099	2088	2066	2200
	Y direction	2035	2150	2156	2171	2015
Height: 2,10 m Size 0,40 × 0,40	X direction	2030	2105	2145	2144	2014
	Y direction	2156	2148	2154	2148	2154
P6 Dx T average ultrasonic speed: 2560,9 m/s						
Height: 0,70 m Size 0,46 × 0,46	X direction	2562	2563	2545	2568	2567
	Y direction	2575	2574	2586	2544	2578
Height: 1,40 m Size 0,43 × 0,43	X direction	2543	2546	2566	2564	2568
	Y direction	2545	2544	2543	2549	2548
Height: 1,40 m Size 0,43 × 0,43	X direction	2566	2531	2533	2540	2545
	Y direction	2562	2545	2565	2555	2551
Height: 1,40 m Size 0,43 × 0,43	X direction	2556	2565	2559	2557	2549
	Y direction	2552	2553	2545	2551	2553

TABLE 2: Continued.

		P6 Dx T average ultrasonic speed: 2560,9 m/s				
Height: 2,10 m Size 0,40 × 0,40	X direction	2565	2596	2586	2567	2568
	Y direction	2564	2572	2575	2574	2573
		2564	2561	2563	2559	2568
		2567	2586	2584	2583	2569
		P1 Sx I p average ultrasonic speed: 2582,0 m/s				
Height: 0,70 m Size 0,46 × 0,46	X direction	1899	2130	2156	2102	2105
	Y direction	2111	2182	2221	2310	2229
		3200	3180	3196	3102	3104
		3016	3038	3086	3048	3062
Height: 1,40 m Size 0,43 × 0,43	X direction	1700	1750	1890	1806	1830
	Y direction	1834	1905	1942	1972	1973
		3109	3140	3149	3131	3158
		3160	3263	3264	3285	3189
Height: 2,10 m Size 0,40 × 0,40	X direction	2005	2103	1999	2150	2015
	Y direction	2060	2160	2163	2184	2150
		3005	3158	3139	3190	3100
		3089	3048	3069	3099	3105
		P1 Dx I p average ultrasonic speed: 3006,5 m/s				
Height: 0,70 m Size 0,46 × 0,46	X direction	3000	3002	3005	3019	2993
	Y direction	3010	3015	3019	2921	2996
		3012	3015	3018	3016	3018
		3001	3021	3029	3028	3020
Height: 1,40 m Size 0,43 × 0,43	X direction	3000	3052	3013	3002	3015
	Y direction	2999	3050	3015	3016	3019
		2996	3047	3036	3033	3031
		3002	3052	3035	3063	3043
Height: 2,10 m Size 0,40 × 0,40	X direction	2899	2915	3005	3001	3045
	Y direction	3015	3062	3036	3035	3046
		3051	3015	3063	3026	3033
		3018	3017	3021	3012	3026

TABLE 3: Left side pillar (Sx): Ground floor.

Pillar	Average rebound value	Average ultrasonic speed
P1 Sx T	41	2862,9
P2 Sx T	43	3297,1
P3 Sx T	34	3345,3
P4 Sx T	32	3181,2
P5 Sx T	30	3192,1
P6 Sx T	36	2134,1

TABLE 4: Right side pillar (Dx): Ground floor.

Pillar	Average rebound value	Average ultrasonic speed
P1 Dx T	28	2873,2
P2 Dx T	38	3103,6
P3 Dx T	32	3199,3
P4 Dx T	38	3313,2
P5 Dx T	34	3082,1
P6 Dx T	36	2560,9

22 MPa, that take in a significant consideration the contribution of the covered concrete conditions.

5. Conclusions

The tests carried out are the basis of a diagnostic project that is possible to implement and monitor to guarantee

a deeper knowledge, with the goal of attaining a level of thorough understanding aimed at the “preservation and improvement” of a building.

The recovery project, that takes particular care in the methodological application of the diagnostic phase, cannot leave out of consideration the necessity of a careful and timely monitoring of building conditions.

TABLE 5: Left side pillar (Sx): First floor.

Pillar	Average rebound value	Average ultrasonic speed
P1 Sx I p	23	2582,0
P2 Sx I p	39	3476,1
P3 Sx I p	36	3358,2
P4 Sx I p	36	3319,6
P5 Sx I p	37	3450,3
P6 Sx I p	38	3346,1

TABLE 6: Right side pillar (Dx): First floor.

Pillar	Average rebound value	Average ultrasonic speed
P1 Dx I p	23	3006,5
P2 Dx I p	31	3210,2
P3 Dx I p	39	3120,9
P4 Dx I p	38	3204,6
P5 Dx I p	38	3098,1
P6 Dx I p	37	3301,5

The carried out tests are the first and simplest analysis for a qualitative assessment; it is necessary to classify the structure regarding the following consolidation procedures. The recovery and conservation project, as well as an “indispensable” transformation of an old building, must be in that evaluations, of feasibility and suitability, both economic and practice which is the basis for a “suitable choice” of recovery intervention, that permit to annul the “cancellation” of the Built Heritage.

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